

16/9/10.

# S.S. Casique

Extracts - from correspondence between Mr. A. L. Turner & Mr. Shanks re above vessel dated July, Aug & Sept. 1910.

Main or 2<sup>nd</sup> deck down in way of No. 3 Hatch aft of Engine, deflected between the hatch end beams.

Hatch 28'2" long with wide spaced quarter pillars & girders between same in addition to flanged coamings. - see plans.

Measured deflections between hatch ends = 3" port 5" starboard  
these deflections being permanent i.e. "set down"

Wt. carried in lower deck space in way of No. 3 hatchway was 586 tons - nitrate of soda in bags - stows at about 30 c/ per ton. -

Assume a normal load of coal on deck @ 50 c/ per ton  
wt on both girders =  $\frac{32 \times 28.16 \times 8.75}{50} = 158 \text{ tons}$ .

Compressive stress on upper edge of hatch coaming angles assuming no assistance or otherwise from 1/2 beams at sides beyond 16 ft from Cr. line. & semi-rigid end attachments  
i.e.  $\text{Stress} = \frac{wl}{10}$  i.e. worst conditions under a normal load =  $\frac{158 \times 338}{10 \times 294} = 18.0 \frac{\text{tons}}{\text{sq. in.}}$

stress at top of plate coaming assuming B.A. failure  
=  $\frac{158 \times 338}{10 \times 334} = 16.0 \frac{\text{tons}}{\text{sq. in.}}$

with 586 tons of cargo at 30 c/ per ton = 17580 c/

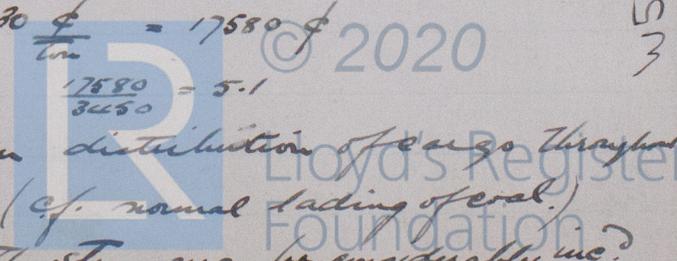
Area of floor 50 x 69 = 3450

$$\frac{17580}{3450} = 5.1$$

Wt. on girders assuming uniform distribution of cargo throughout  
Compt. =  $\frac{32 \times 28.16 \times 5.1}{30} = 153 \text{ tons}$  (cf. normal loading of coal)

Since with any other distribution the stress may be considerably inc. in direct proportion to the depth of cargo over the girders. -

3598-0281 1/4



without BA.

58.62

320.9

310

11.76

5.47

1700

5.47

6.20

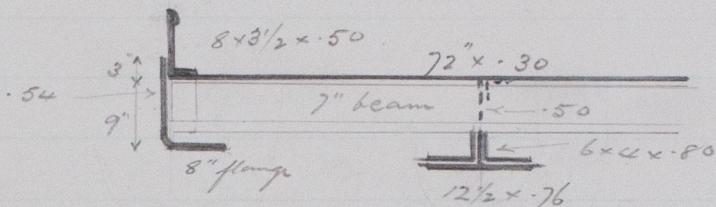
1410

6.29

8.47

224

min 3 bottom }  
2 top }



Coaming	12 x .54	6.48	4.5	29.2	131	78
flange	8 x .54	4.32	8.73	37.7	330	
web	4 x .50	2.00	9.0	18.0	162	2
2 Angles	6 x 4 x .80	14.72	8.89	131.0	1162	17
face plate	12 1/2 x .76	9.50	11.38	108.0	1228	
deck	72 x .30	21.60	-.15	-3.24	-	
BA	8 x 3 1/2 x .50	6.25	-3.8	-23.8	90	50
		64.87		-27.0	3108	147
				1296.9	147	
				4.58	3250	
				7.18	1360	
					1890	

11.76  
4.58  
7.18

25.5  
10  
25.7

4.58  
8.3  
12.88

$I_y = 263$  min 3 bottom }  
 $I_y = 147$  top }

With double 12 x 4 x 4 x .64 Channels in lieu of double angles face plate.

Channels	24.22	13.0	394	5115	470
Web 12 x .5	6.00				
	30.22		394	5657	
less 111 ashore	26.22		257	2571	
diff =	4.00		137	3086	
Add ashore	64.87		296.9	3250	
	68.87		433.9	6336	
			6.3	2725	
			12.7	3611	

$I_y = 284$  min 3 bottom }  
 $I_y = 247$  top }

Note:  $I_y$  of III = 202 (bottom)  
 $I_y$  of II = 237 (bottom)



© 2020

Lloyd's Register Foundation

W598-0281 2/4

Assume long strips of deck 32 ft wide & 28.16 ft span  
i.e. no work done by half beams.

$$\text{Q of cargo } 586 \times 30 = \underline{\underline{17580 \text{ q.}}}$$

$$50 \times 69 \times 8 = \underline{\underline{27500}}$$

$$\text{Area floor } 50 \times 69 = \frac{17580}{3450} = 5.1 \text{ ft}^2 \text{ wt of cargo assuming uniform distribution}$$

$$\text{Area supported by girders} = 28.16 \times 32 = 900$$

$$\text{wt} \quad \quad \quad = \frac{900 \times 5.1}{30} = 153 \text{ tons}$$

$$\text{Stress of top edge of girders assuming simple supports} = \frac{153 \times 28.16 \times 12}{8 \times 147 \times 2} = 22 \frac{\text{wt}}{\text{ins}^2}$$

$$\text{"do" bottom} \quad \quad \quad = \frac{26 \times 147}{203} = \underline{\underline{12.3}}$$

Assuming semi-fixed supports stress is

$$\text{top } \frac{26 \times 4}{5} = 17.6$$

$$\text{bottom } \frac{123 \times 4}{5} = \underline{\underline{9.8}}$$

$\frac{\text{wt}}{10}$

Assuming worst case i.e. no



© 2020

Lloyd's Register  
Foundation

W598-0281/4

Total weight in Compartment as per Int-Tarans



© 2020

Lloyd's Register  
Foundation

Hatches off upper DR

$$.17 = \frac{wL}{10 \times 2}$$

Take El. L<sup>1</sup> @  $17 \frac{\text{in}}{\text{min}^2}$  & B.M. =  $\frac{wL}{10}$

$$\text{Ly of both sides} = 147 \times 2 = 294$$

$$\text{wt to strain to El. L<sup>1</sup>} = \frac{.17 \times 10 \times 294}{350} = 143 \text{ lbs.}$$

$$\text{def}^n = \text{load}^d = \frac{3}{384} \frac{wL^3}{EI}$$

$$= \frac{3 \times 143 \times 294^3}{384 \times 13,000 \times 3750} = .98''$$

load on girders with 8.75 ft of coral @ 51d

$$\rightarrow \frac{32 \times 28.16 \times 8.75}{50} = 158 \text{ lbs.}$$



© 2020

Lloyd's Register  
Foundation

W598-0281 4/4